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Statistical Search for High Water at Board Landing Bridge, Truro, Nova Scotia

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STATISTICAL SEARCH FOR HIGH WATER AT BOARD LANDING BRIDGE, TRURO, NOVA SCOTIA

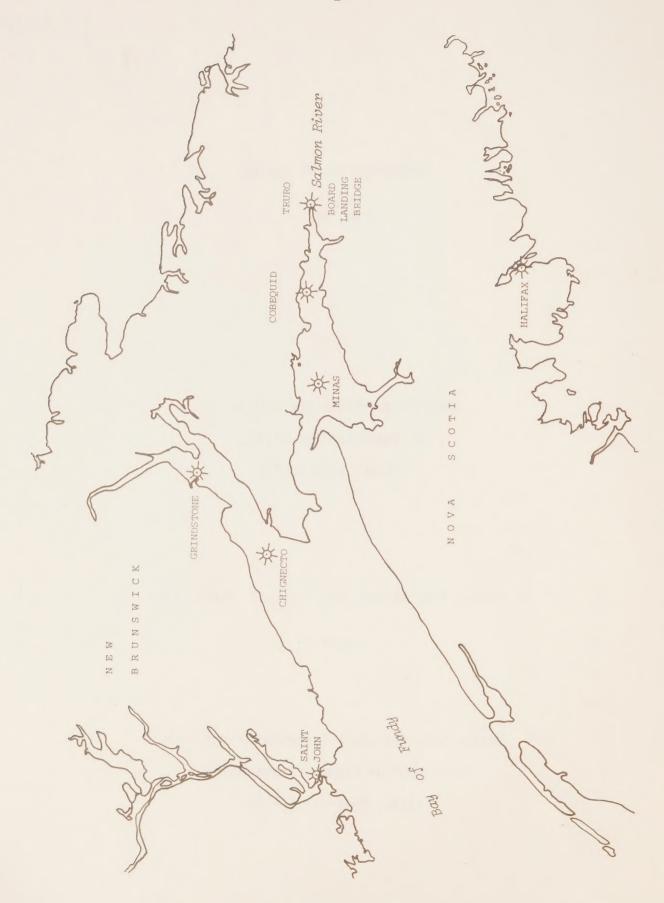
G. Godin, P.A. Bolduc, D.G. Mitchell and S. Yuen

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1. ABSTRACT

A small set of intermittent tidal observations on the height of high water at the Board Landing Bridge is scrutinized in order to estimate a height for the highest high water likely to occur there. The random portion of the signal is evaluated following a regression analysis with Saint John and other stations. The predicted height of high water at Saint John gives the best fit, suggesting little coupling between nontidal events at the two sites. A height of 33.3 ft with a return period of about 90 years is inferred.

RÉSUMÉ

Un petit nombre d'observations intermittentes sur la hauteur de la pleine mer au site du pont Board Landing est étudié afin d'estimer la hauteur de la plus forte pleine mer qui pourrait y être sentie. On évalue la portion aléatoire du signal à l'aide d'une analyse de régression avec Saint Jean et d'autres stations. La hauteur prédite de la pleine mer à Saint Jean donne les meilleurs résultats impliquant qu'il y a peu de corrélation entre les événements autres que la marée aux deux sites. On infère une hauteur de 33.3 pieds avec une période de retour de 90 ans.



Board Landing Bridge, Tidal Bore Road, Truro County, Nova Scotia (FGB photo, 1981).

2. INTRODUCTION

In an effort to obtain an estimate of the height of the high water at Board Landing Bridge (hereafter BLB) which has little probability of being exceeded, we carried out a statistical search of the existing, but small (two years only) body of tidal observations there (frontispiece). Strong shallow-water distortion in the area precluded the use of a direct harmonic analysis, so that we calculated correlations between the time of travel of high water to BLB and its height there, against the height of high water at a number of nearby "reference" stations as shown in the following:

Saint John (observed and predicted time and height of high water),
Chignecto (predicted, using 81 days of observations),
Grindstone (predicted from 87 days),
Minas (predicted from 86 days) and
Cobequid (predicted from 87 days).

Regressions were also made between the observed height and time of high and low water at Minas and Cobequid against Saint John. The results of previous investigations are summarized in the report C.D. 77.3.9-1 compiled by DILLON (Consulting Engineers and Planners): Tides in the Cobequid Bay and the Salmon River Estuary.

We have long series of reliable observations for Saint John but its distance from BLB suggested that we test the other sites as references, e.g. the submerged-gauge stations set up in the upper Bay of Fundy during 1976. These are more representative of the tide in the basin and in particular Cobequid monitors the wave just as it enters Salmon River. (The submerged gauges were not in operation during 1971-2). Harmonic constants were obtained from the observations; these being almost noise-free we could obtain reliable estimates of all the major constituents by superresolution. Predictions were prepared for the time interval of interest.

We retained only Saint John eventually because the observed extremes fell close to the regression curve, while those for the submerged stations strayed. Weakness in the predictions for the submerged stations and irregularities in the original recording at the Cobequid site were the probable causes for this erratic behaviour.

Once Saint John was retained and the regression verified, we checked to determine if there was strong coupling between events of nontidal origin at Saint John and BLB. The regression with the predicted high water at Saint John gave slightly better results than with the observed high water; this suggests weak coupling between the two sites for nontidal events. An additional inspection of extreme tides both at Saint John and BLB suggests also that deviations from the predicted tide at Saint John are not strongly correlated with deviations at BLB from values predicted from the regression formula.

We conclude that we may obtain an estimate of the height of high water

at BLB from the predicted high water at Saint John and the local extreme from the observed deviations from the regression. This was done and is presented in a later section.

The tide in the Bay of Fundy is of the perigean type (larger when the moon is nearer, i.e. every 27.55 days) because the frequency of the component, N2, (created by the variation in the moon's distance) falls very near the resonant frequency of the bay (Godin 1980). The major lunar and solar components M2 and S2 are also amplified; their interference creates spring and neap tides. Whenever spring tides coincide with perigean tides, the largest tides of the year occur in the Bay of Fundy. This corresponds to the astronomical situation in which the moon is in perigee during new or full moon.

The resonance conditions are different inside Minas Basin: M2 is further amplified, while N2 is nearly constant. The dynamic response of the main body of the Bay of Fundy and of Minas Basin to the same impulse must be quite different. Some chance coupling between the two on the other hand might create a dramatic response of the whole system. Friction becomes important in the upper reaches of the bay and the tide takes eventually the form of a free travelling wave increasingly distorted by friction and is eventually stopped by it.

Only high water is felt at BLB as the water during ebb recedes a few kilometers away. The tide seen there is the last breath of a once young and healthy signal passing by Saint John. It may easily be perturbed by meteorological conditions and the records of high water should be fairly irregular even if accumulated with the greatest care. In the absence of events of meteorological origin, the worst conditions for flooding will be created when perigean and spring tides coincide and the river is undergoing a maximum discharge. Higher levels downstream impede the flow and create higher level upstream even where no tide is felt.

3. DATA PROCESSING

The basic data consist of discontinuous hourly observations on the height of the water level at BLB taken by Water Survey of Canada during some months of 1971 and 1972. The observations cover the approximate time of high water although they do miss it occasionally. We applied Lagrangian interpolation to the data in order to extract the time and height of local high water; using the material available and rejecting unuseable data, we were left with 315 times and heights of high water, these forming a more or less adequate sample for further investigation.

We wish to represent the height, H, of the high water at BLB and the time, Δt , it took to reach from the reference station in terms of the height of high water at the reference station by:

$$\hat{y} = a + b\hat{x}$$

where \hat{y} is the height (feet) of high water at BLB or the time (hours) it took high water to reach it, a the y intercept, b the regression coefficient and \hat{x} the height of high water at the reference station (predicted or observed).

This is a least square fit and one needs some statistics to evaluate its representativeness.

Variable x: mean \overline{x} standard deviation s_{χ}

Variable y: mean \overline{y} standard deviation s_y

The standard error of the estimate s_F :

$$s_{E} = \sqrt{\sum_{i=1}^{n} \frac{(y_{i} - \hat{y}_{i})^{2}}{n-2}}$$

where y_i is the actual value of the i^{th} sample and \hat{y}_i is its value deduced from the regression. It is a measure of the goodness of fit between the assumed straight line relation and the actual data.

The confidence interval of the regression coefficient is:

$$\pm t_{(\frac{1}{2}(1+g)),n-2]}s_b$$
 at 100g% confidence level.

Choosing 99% implies g=.99. This gives $\frac{1}{2}(1+g)=.995$ n-2 = 313, so that $t_{[.995,313]} \sim 2.59$ from the student t distribution.

$$s_b \equiv \frac{s_E}{\sqrt{\sum_{i=1}^{n} x_i^2}}$$

The reduction in the sum of squares due to the regression is:

$$R^{2} \equiv n\overline{y}^{2} + b \sum_{i=1}^{n} x_{i}y_{i}$$

The % of variation in the sum of squares contributed by the regression is:

$$\frac{R^2}{\sum_{i} y_i^2} \times 100$$

The residual sum of squares is $_{i}^{\Sigma}$ (y_{i} - y_{i})² and its mean values is s_{E}^{2} . The hypothesis of 0 regression is tested with the t test and the F test. The t test consists in forming the variable:

$$t = \frac{b - B}{s_b}$$

where b is the calculated regression coefficient and B is the true coefficient hypothesized to be 0. The hypothesis is false at 100g% probability if:

$$t \equiv \frac{b}{s_b} > t_{\frac{1}{2}}(1+g), n-2$$

at 99% t.995.313 ~ 2.59

at 99.9% t.9995,313 ~ 3.32

The F test consists in forming:

mean square explained by the regression residual mean squares

The hypothesis of 0 regression implies F = 1. The hypothesis is rejected if at 100g% probability:

$$F > F_{g,1,n-2}$$

at 99% F.99,1313 ~ 6.63 at 99.9% F.9995,1313 ~ 12.5

In the plots, the limits of prediction are given by the curves:

$$\begin{cases} U = 9 \pm t_{\frac{1}{2}}(1+g), n-2 \end{cases}$$

at 100g% confidence, where:

$$s_{\hat{y}} = s_E \sqrt{1 + \frac{1}{n} + \frac{(\hat{x} - \overline{x})^2}{\sum_{i} x_i^2}}$$

R being a regular sequence of values along the abscissa. The printouts in the appendix give all the statistics mentioned:

> variable 2 x: average value \overline{x} standard deviation s_x variable 1 y: average value y standard deviation s

the correlation coefficient between the two variables:

$$r = \frac{\sum_{i}^{\Sigma} (x_{i} - \overline{x})(y_{i} - \overline{y})}{\sqrt{\sum_{i}^{\Sigma} (x_{i} - \overline{x}) \sum_{i}^{\Sigma} (y_{i} - \overline{y})^{2}}}$$

the number of samples, n, the regression coefficient, b, the standard error of the regression coefficient:

$$\frac{s_E}{\sqrt{n} s_X}$$

the computed t value b/sb for the t test, the proportion of variation:

$$R^2 \times 100/\Sigma y_i^2$$

the residual sum of squares and its mean values s_E^2 the y intercept, a, the standard error of the estimate, s_E , the F level, mean square explained by the regression/residual mean square, the contribution of the regression to the sum of squares (1 degree of freedom) and the residual sum of squares.

We see from Appendices 4 and 5, that all the regressions with the chosen reference stations pass the t and F tests to a high degree of reliability; therefore any one of the stations chosen is suitable as a reference station. The inspection of the results will indicate which one is optimum. The results of the statistics are summarized in Table 1.

Table 1 indicates that Saint John is the best predictor for the heights. The correlation for the pure tide as input (Saint John predicted) is a shade higher than for the actual level recorded (Saint John observed). The difference (.004) in the correlation coefficient is not statistically significant but an inspection of the scatter diagrams (Figure 1) indicates that Saint John (predicted) contains fewer outliers in the region of extreme levels than Saint John (observed). We conclude that the predicted tide at Saint John (Saint John (p)) is a better predictor of the height of high water at The weaker correlations for the submerged stations are mainly due to the fact that shallow-water distortions become appreciable in Minas Basin (Minas and Cobequid) and that standard harmonic predictions fail to predict adequately the height of extremes. The better correlations obtained for stations in the Chignecto arm of the bay (Grindstone and Chignecto) suggest that the harmonic analysis of these records has been more successful in reproducing the levels actually observed. All of these can be discarded as predictors of the height of high water at BLB, including Cobequid the nearest available station to BLB.

The coefficient of regression on the times (Figure 2) is negative in all cases (see also printouts in the appendix and Table 1), indicating that the velocity of the free wave reaching BLB increases with the height of high water. The correlation values follow our intuition this time, Cobequid appearing as the best predictor. We obtain a few seemingly absurd values for Cobequid: some points indicate the tide reaches BLB before Cobequid. Since these latter values are of the order of 0.1 to 0.2 hours, they fall within

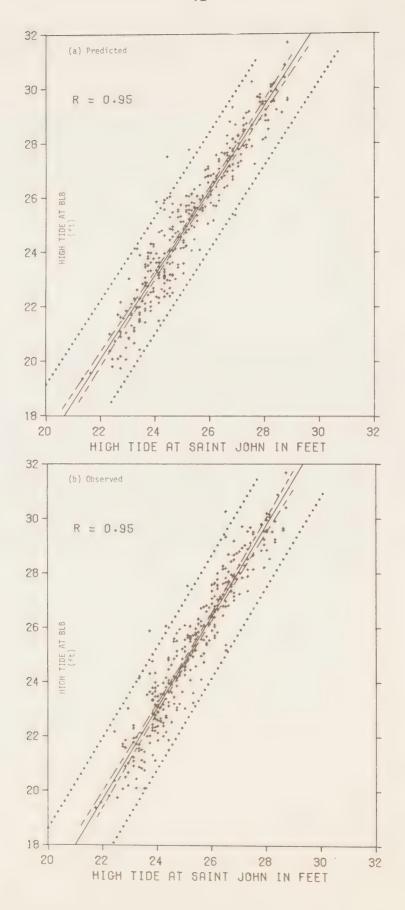
Table 1. Summary of the regression calculations arranged in decreasing order of correlation with respect to the heights.

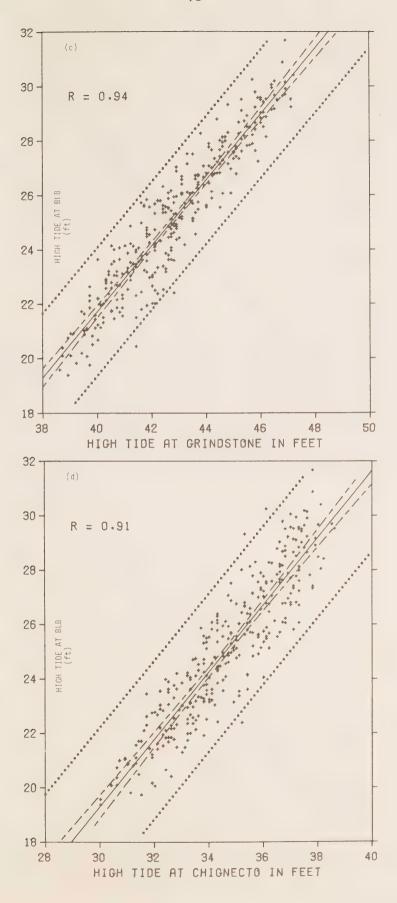
Reference	Height of	High Water at BLB	Time Difference			
Station	Correlation r	Regression Equation (ft)		Regression Equation (hours)		
Saint John (predicted)	.950	H = -14.024+1.550Hpred	247	$\Delta t = 2.321030H_{SJ}^{pred}$		
Saint John (observed)	.946	$H = -17.126 + 1.673 H_{SJ}^{obs}$	235	$\Delta t = 2.295030 H_{SJ}^{obs}$		
Grindstone (predicted)	.941	$H = -26.611 + 1.208 H_{G}$	481	$\Delta t = 3.562053 H_{G}$		
Chignecto (predicted)	.908	$H = -17.689 + 1.233 H_{C}$	443	$\Delta t = 3.458062 H_{C}$		
Cobequid (predicted)	.901	$H = -29.980 + 1.073 H_{Cob}$	650	$\Delta t = 4.490079 H_{Cob}$		
Minas (predicted)	.877	$H = -30.291 + 1.184 H_{M}$	632	$\Delta t = 4.519084 H_{M}$		

Table 2. Time necessary for high water to reach BLB for various values of the height of high water at the reference station using the regression relation.

Reference Station	Height of High Water at the Reference	Time Needed for High Water to Reach the Site
	(ft)	(h)
Saint John	22 24 26 28 30 32	1.64 1.58 1.53 1.47 - 1.41 1.35
Minas	43 45 47 49 51 53	.93 .77 .60 .43 .27
Cobequid	4 7 49 51 53 55	.78 .62 .46 .30 .15

Fig. 1. Regressions between predicted or observed heights at stations in the area and at BLB. The stars are the samples, the solid line is the regression straight line and the dashed lines are the limits of the regression straight line at 99% of the samples. The dotted lines should contain 99% of the samples. The coefficient of correlation is given in the upper left corner. a) Between the predicted height of high water at Saint John and the observed height of high water at BLB. b) Between the recorded height of high water at Saint John and the observed height of high water at BLB. Note the samples fall close to the regression curve in the region of extremes for the predicted height at Saint John and the more even distribution of stray points for Saint John predicted. c) Between the predicted height of high water at BLB. d) Between Chignecto and BLB. e) Between Cobequid and BLB. f) Between Minas and BLB.





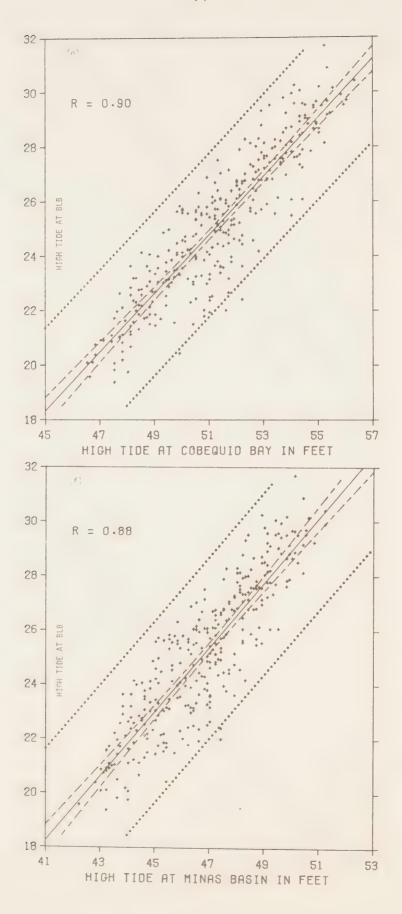
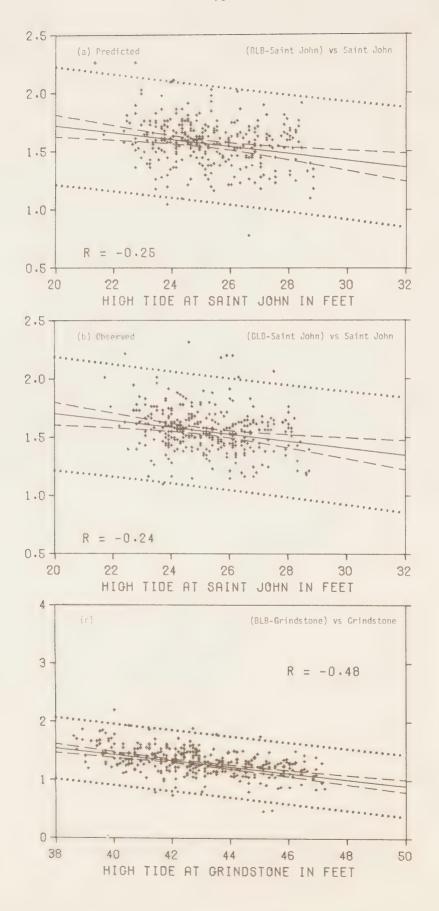
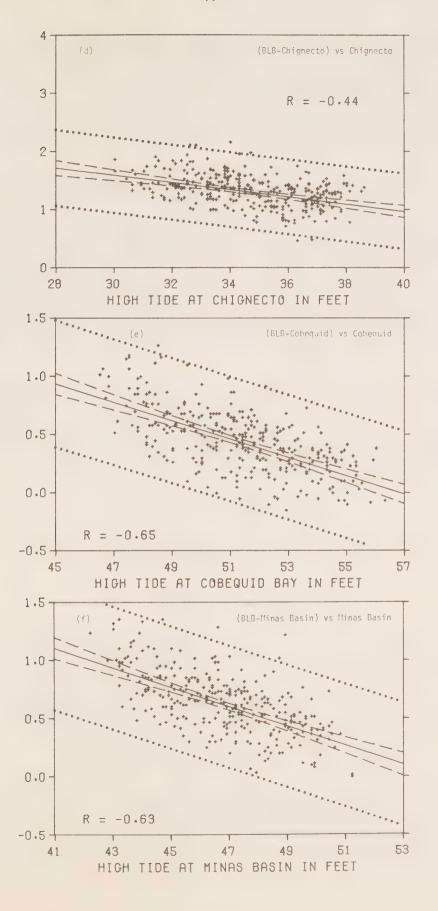


Fig. 2. Regressions between the predicted or observed height of high water at stations in the area and the time difference between the occurrence of high water at the particular location and BLB. A negative slope indicates that the higher the high water the faster it progresses toward Salmon River, i.e. toward BLB. a) Between the predicted height of high water at Saint John and BLB. b) Between the observed height of high water at Saint John and the time interval. Note again the use of Saint John predicted as input results in a more natural distribution of sample points. c) Regression with Grindstone. d) Regression with Chignecto. e) Regression with Cobequid. f) Regression with Minas.





the limits of tolerance of the predictions we can prepare for the data at Cobequid. We calculate (Table 2) the mean time it takes for the high water to reach BLB and the velocity of the tidal wave for various heights of high water at the reference stations.

Table 2 makes it quite clear that there might be dramatic changes in the velocity of progress of the tidal wave during very high tides.

4. SEARCH FOR HEIGHT OF HIGHEST HIGH WATER

The high water reached at BLB is the superposition of a pure tide signal and of an additional displacement contributed by the response of Minas Basin to local winds and pressure fields, and to disturbances coming from the Bay of Fundy. The height of high water due to the tide has a mean and standard deviation but it definitely is not a random variable: it has a well-defined upper bound determined by the combination of the tidal constituents producing the largest possible tide. The disturbance in level may be treated as a random variable whose upper bound can be ascertained at a given confidence level with the help of statistics.

We cannot dissociate directly the pure tide at BLB from other perturbations, but we may attempt to do so with the help of the statistics just obtained from comparisons with Saint John. The question is: is a disturbance at Saint John automatically reflected in the level at BLB? We therefore review extreme events which can instruct us about the association or lack of association between nontidal events at Saint John and BLB:

- a) levels exceeding 30 ft at BLB,
- b) extreme high tides recorded at Saint John during 1971-72 and their repercussion at BLB, and
- c) the extreme level recorded at Saint John.

4.1 Scrutiny of extreme levels at BLB and Saint John

We go over the levels exceeding 30 feet at BLB in order to determine whether:

- a) they differ appreciably from the regressed values and
- b) if so, whether Saint John was disturbed during the event and in what way?

Table 3 gives the observed level at BLB, the regressed level using Saint John predicted, their difference, the regressed level using Saint John observed, their difference, the time and height of the predicted high water at Saint John, the time and height of the observed high water at Saint John and finally the height difference between the predicted and observed high water. The last value indicates whether Saint John was perturbed or not during the high level at BLB.

Table 3 suggests that although the extreme of 31.6 ft at BLB seems at first as the most noteworthy, the later extreme occurring at 14 hours involves an even larger local disturbance. Three events appear to reflect a true disturbance inside Minas Basin: the one at 15 hr 7 October, 1971 and the two just mentioned which occurred 15 April, 1972. The tide at Saint John was unperturbed during the first event. On the 15 April 1972 the first big tide at Saint John was completely normal while the level in Minas Basin was significantly abnormal; we have a situation in which a strong but normal tide at Saint John is associated with a disturbance in Minas Basin. Saint John was significantly perturbed at 14 hours: its level was significantly lowered while the level at BLB was abnormally raised.

Table 3. Levels exceeding 30 ft at BLB in regards to the possible occurrence of events at Saint John.

Time and Date	Level Observed	Saint John Pred	gressed .Diff.	Saint John Obs.	Diff.	0bse		Pred.	icted	Height Difference
(h, d, m, y)	(ft)	(ft)	Δ	H (ft)	Δ	Time	Height (ft)	Time	Height (ft)	(ft)
1.0 26/04/71	30.4	30.0	.4	30.9	5	23 48	28.7	23 50	28.4	.3
14.9 07/10/71	30.1	29.2	.9	29.9	.2	13 22	28.1	13 20	27.9	. 2
12.8 03/11/71	30.8	30.6	.2	30.4	. 4	11 21	28.4	11 20	28.8	4
.9 14/04/72	30.1	29.7	.4	29.7	4	23 20	28,.0	23 20		he2
1.5 15/04/72	31.6	30.5	1.7	30.9	.7	00 07	28.7	00 05	28.7	3011
14.1 15/04/72	30.2	28.6	1.6	27.2	3.0	12 39	26.5	12 35	27.5	-1.0
14.0 24/10/72	30.0	29.7	.3	29.9	.1	12 26	28.1	12 25	28.2	1

We now review the extreme tides at Saint John during 1971 and 1972 and present (Table 4) the extreme high tides observed at Saint John and the corresponding observed levels at BLB. The table suggests that during these extreme tides, things were regular at Saint John with the exception of 21 and 22 November 1972 when the level was somewhat depressed. We detect a significant abnormality at BLB on the 4 November 1971, 15 April 1972 and 12, 13, 15 May 1972. The height was lower than expected in three events and it was higher in one, while in the mean time, everything seemed to be going fine at Saint John.

We consider finally the extreme level at Saint John. Our records indicate that the highest instantaneous level at Saint John is: 30.25 ft. It occurred at 00 h 15 min 6 April 1977; the predicted high tide was 00 h 45 min and 28.1 ft. The actual high tide therefore was 30 min early and 2.1 ft higher. This must have been created by a major disturbance since, as we have seen, the tide at Saint John comes like clockwork. The Bay of Fundy appears

Table 4. Search for coincident abnormalities in the level at Saint John and at BLB, and in the time taken by high water to reach BLB for extreme tides at Saint John predicted for the years 1971 and 1972.

	Difference ^A (ft)		. 4		. 2
	Difference Regressed Height Saint John Observed A Hg (ft) (ft)		29.7 30.4 30.2	29.7 30.9 30.7 31.1	29.5 29.9 29.7 29.7
Height	Difference A (ft)		1.7	4. 1.1 7 0.1.	0
	Height Regressed Height Observed Saint John Pred. H (ft) (ft)		30.2 30.6 30.2	29.7 30.5 30.2 30.6	29.5 29.7 30.2 30.3
818	Height Observed H		29.5	30.1 31.6 29.5 29.6 29.1	29.5 30.0 29.5 29.9
	Difference		10	.08 06 30 28	. 23
Time	Regressed Value Using Saint John Pred.		1.47	1.48 1.46 1.47 1.46	1.48 1.47 1.46
i.	Time Taken to Progress from Saint John At (h)		1.42	1.56 1.40 1.17	1.53 1.57 1.75
	Time of High Water T (h)	× × × ×	12.0	24.9 1.5 25.0 1.9	13.2
_	Olfterance in Height A (ft)	4 4 4	4	2 - 2 2	S - 1 1 1 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1
SAINT JOHN	Produced Time Height T H	23 05 28.3 23 50 28.4 11 40 28.5 12 30 28.5	10 30 28.5 11 20 28.8 12 10 78.5	73 20 28.2 00 05 28.7 22 55 28.5 23 45 28.8 00 40 28.6	11 40 28.1 12 25 28.2 11 20 28.5 12 10 28.6
	Observed Time Height T H (ft)		10 35 28.0 11 21 28.4 12 11 28.3	23 70 28.0 00 07 28.7 22 56 28.6 23 49 28.8 00 42 28.3	11 40 27.9 12 26 28.1 1 18 28.0 12 09 28.0
	Date	1971 April 24 25 Oct 5	No v S S S S S S S S S S S S S S S S S S	1972 April 13 7 15 C May 12 8	Oct 23 1

as a finely tuned system which responds most selectively to an applied disturbance; its response to the proper input is bound to be spectacular.

We wish to investigate if this level was created by local conditions and if it can be used to calculate an extreme height at BLB. The only local source of disturbance at Saint John which is abutting directly against the Bay of Fundy is the Saint John River; we must check if it was abnormally raised thus causing the abnormal reading. We list (Table 5) the observed and predicted height of high water before and after the event.

Table 5. Observed and predicted height of high water before and after the highest level ever recorded at Saint John.

Date 1977	Observed (ft)	Predicted (ft)	Difference (ft)
April 3	27.5 27.4	26.5 27.1	1.0
April 4	26.5 27.7	27.1 27.8	6 1
April 5	28.1	27.4	.9
April 6	30.0* 27.9	28.1 27.3	1.9
April 7	7 28.1 26.5	28.1 26.8	0 3

^{*}smoothed hourly value

Flood conditions would have caused a persistent rise in levels during at least a few days; the abnormally raised levels lasted some 36 hours. This inclines us to the notion that the event took place in the Bay of Fundy. The weather records indicate moderate winds and somewhat rough seas but nothing of a hurricane nature. The response of the Bay of Fundy to such mild stimulus to create this unusual event deserves to be investigated further. We have no records at BLB for that date.

5. HEIGHT OF THE HIGHEST HIGH WATER USING THE EXTREME LEVEL AT SAINT JOHN

Using conventional regression statistics the maximum height for BLB is: $y_d = \hat{y}(30.25) + t s_0$

where the value of t is determined by the level of confidence chosen and the number of samples. For 315 samples (313 degrees of freedom) and levels of 99.9 and 99%.

t = 3.32 and 2.59

This gives:

y_d = 35.7 ft, 35.1 ft using the prediction regression, and

= 36.3 ft, 35.6 ft using the observed regression

at 99% and 99.9.

These values are based on the assumption that all the samples have equal probability. In fact the extreme events are determined by a compound probability: we need a high tide and an abnormally high disturbance in the level. We can evaluate the probability of high water exceeding a given level at Saint John and we can study the residues from the regression to determine their probability distribution and the return period of the extreme. In this way we attempt to calculate heights corresponding to given return periods.

5.1 Tides of 29 ft and over at Saint John

As derived from analyses of annual records of level, the amplitude of the major constituents of the tide at Saint John are:

M₂ 9.94 ft

S₂ 1.63 ft

N₂ 2.05 ft

0₁ 0.37 ft and

K₁ 0.50 ft.

In addition there are a larger number of constituents of lesser importance. The highest tide would occur if they were all in phase: this is never the case. We may assume that generally the smaller constituents cancel each other and the larger constituents may reinforce or weaken each other.

Perigee tides occur when M₂ and N₂ are in phase; spring tides when M₂ and S₂ are in phase. If the three are in phase, we have a coincidence of perigee and spring tides. The reference level being 14.56 ft, perigee tides would give a high water of 26.6 ft and spring tides, a high water of 26.1 ft on average. Spring perigee tides would give 28.2 ft. Because of the diurnal inequality, K₁ and O₁ increase one of the high waters and decrease the subsequent one; this would give a higher high water of 29.1 ft during spring tides, which should be a very rare event and one whose probability of occurrence we should investigate. Tidal forces have cycles of a month, a year, 9 years, 18 years and 26,000 years; the 26,000-year cycle is too long to be

perceptible in our records, but the 9-and 18-year cycles are certainly noticeable. Astronomy instructs us that the semidiurnal forces had an 18 year maximum in 1960 and 1978; the next peak is due in 1997 (Anonymous 1967). It will coincide with a minimum of the diurnal forces; therefore we cannot expect the largest tide ever to occur on that very year. If we inspect the tide tables for Saint John, we note predicted heights of high water of 29.1 ft on 4 April and 6 April 1958; the next highest high water is 29.1 ft 3 June 1977. Therefore the actual tide maximum in the Bay of Fundy occurs some one or two years before the peak in the semidiurnal forces and may be due in part to the accidental additional contribution of some of the lesser constituents.

In order to check on the interval of reoccurrence of extreme tides in the Bay of Fundy we prepared one hundred years of predictions covering the interval 1981 to 2080 for Saint John. We give (Table 6) the 100 highest tides arranged in chronological order and in magnitude (there are many more 28.7-ft tides than indicated since the computer was ordered to stop at the 100th entry). We notice that the absolute highest tide is indeed 29.1 ft and occurred 8 times in 100 years; one should be tempted to give it a probability of once every twelve years. But the tide being a deterministic signal, it follows the force applied and not the rules of dice. In fact we have two 29.1-ft tides on 6 and 8 May 2016 or twice in 24 hours, then another the following year on 26 May 2017. The next 29.1-ft tide occurs 17 May 2034; we therefore have four peak tides in 18 years. We note in passing that extreme tides in the Bay of Fundy occur either at midnight or at noon, the midnight tide occurring in spring and the noon tide occurring during autumn. Since both seasons are storm prone, we may expect an increased probability of catastrophic conditions during these extreme tides. We note also that there is a secular increase in the mean level along the Atlantic seaboard of 0.01 ft/ year (Dohler and Ku 1970) which has not been taken into account because it is impossible to predict its persistence or lack of persistence in the future. Returning to our problem of extreme heights at BLB we recall that the observed extreme local level of 31.6 ft corresponded to a tide of 28.7 ft at Saint John. Therefore in the practical situation the existence of extreme local levels seems to be linked to large tides in the Bay of Fundy and not to specific extreme tides which exceed the average large tide by only a few tenths of a foot.

An example is the "Saxby Gale" which occurred 4 October 1869 (Hutchinson 1912; Crane 1969). Tide "hindcasting" for the day gives a high tide of 28.0 feet at 22:35 hours, a level which is more than one foot below the absolute highest tide. Nevertheless, around that time of that day, the Saint John area was subjected to one of the greatest storms and highest water levels recorded, if not in data records, at least in memory as being very destructive.

"The tide exceeded in height any previous record, the water dashing over all of the wharves, tearing vessels away from their moorings, wrecking them on the beaches, and damaging others by pounding them against wharves. The damage was not confined to Saint John. At Point Lepreau the bark Genil was

Table 6. One hundred maximum tides at Saint John between 1981 and 2080 showing date, time and maximum level (WLMAX) in feet. The list of j harmonic components used to prepare the table is given in appendix 1. The list was set up by studying the results of a succession of analyses and deducing what seemed the most plausible values of the constituents at Saint John. There is an element of subjectivity in this, especially in the case of L2 and μ_2 which vary appreciably from year to year. The predictions obtained from this set will differ in some details from those found in tide tables for which the latest constituents are used as a base for predictions.

CC1 04/05/1981 23:50 29:1 002 27/04/1994 00:10 29:1 CC3 C5/11/1998 12:105 29:1 004 15/05/1999 23:45 29:1 CC3 C5/05/2017 00:100 29:1 008 17/05/2034 23:50 29:1 CC7 26/05/2017 00:100 29:1 008 17/05/2034 23:50 29:1 CC9 06/05/1981 00:50 29:0 010 30/03/1998 00:45 29:0 CC9 06/05/1981 00:50 29:0 010 30/03/1998 00:45 29:0 UT1 17/705/1999 07:25 29:0 012 U39/04/2016 00:35 29:0 UT1 17/705/1999 07:25 29:0 014 16/11/2016 12:40 29:0 UT1 17/705/1999 07:25 29:0 014 16/11/2016 12:40 29:0 UT1 17/705/1998 00:10 29:0 016 25/11/2034 11:20 29:0 UT1 10/05/2052 00:50 29:0 020 01/05/2056 00:20 29:0 UT1 207/04/2038 U0:40 29:0 016 18/11/2051 11:00 29:0 UT1 01/11/2073 12:15 29:0 020 01/05/2056 00:20 29:0 UT1 01/11/2073 12:15 29:0 020 01/05/2056 00:20 29:0 UT3 01/05/2052 00:50 29:0 020 01/05/2056 00:20 29:0 UT3 01/05/2052 00:50 29:0 020 01/05/2074 23:55 28:9 UT3 01/05/2074 23:55 29:0 024 04/11/1998 11:35 28:9 UT3 01/05/2074 23:15 28:9 026 28/04/1998 00:35 28:9 UT3 01/05/2074 23:15 28:9 026 28/04/1998 00:35 28:9 UT3 01/05/2074 23:15 28:9 028 04/11/1998 12:50 28:9 UT3 01/05/2074 23:15 28:9 028 04/11/1998 12:50 28:9 UT3 01/05/2074 23:15 28:9 030 10/06/2016 01:15 28:9 UT3 01/05/2074 23:15 28:9 030 10/06/2016 01:15 28:9 UT3 01/05/2074 23:15 28:9 030 10/06/2016 01:15 28:9 UT3 01/05/2074 23:15 28:9 030 10/06/2018 12:10 28:9 UT3 01/05/2073 11:25 28:9 034 07/11/2033 12:20 28:9 UT3 01/05/2073 11:25 28:9 034 07/11/2033 12:20 28:9 UT3 01/05/2073 12:55 28:9 038 28/10/2038 12:10 28:9 UT3 01/05/2073 12:50 28:9 036 28/11/2034 12:15 28:9 UT3 01/05/2073 12:50 28:9 036 28/11/2034 12:15 28:9 UT3 01/05/2073 12:50 28:9 036 28/11/2034 12:15 28:9 UT3 01/11/2051 12:00 28:9 044 29/04/2052 23:50 28:9 UT3 01/11/2054 11:55 28:9 036 28/11/2034 12:15 28:9 UT3 01/11/2054 11:55 28:9 036 28/11/2034 12:10 28:9 UT3 01/11/2054 11:55 28:9 036 28/11/2034 12:10 28:9 UT3 01/11/2054 11:55 28:8 036 28/11/2034 12:10 28:9 UT3 01/11/2054 11:55 28:8 036 03/10/2033 12:10 28:9 UT3 01/05/2070 02:20 28:8 036 03/10/2039 12:10 28:8 UT3 01/05/2070 02:20 28:8 036 03/10/2033 12:10 28:8 UT3 01/05		DD/MM/YYYY	нн:мм	WLMAX		DD/MM/YYYY	нн:мм	WLMAX
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	0,7,5	53/00/1305	23.40	∠8 • 7	100	06/05/1985	00:05	28.7

wrecked and 11 lives lost. In Albert County the tides caused damage estimated at \$250,000, and Westmorland had the highest tide ever known, the water at Moncton rising six and a half feet above previous records. Buildings were blown away and smashed into bits." (Crane 1969).

Hutchinson (1912, p. 256) stipulated that low atmospheric pressures (barometer 29.3 inches of $H_{\rm g}$) were recorded around that time. The combined effects of the storm surge and the tidal wave produced a level which exceeded the absolute highest tide level.

5.2 Study of residuals from the regression with Saint John

We give (in the appendix) the differences between the observed height at BLB and the regressed height (using the Saint John high water as the predictor) and also the error for the time of travel of high water. These discrepancies are given for the observed and the predicted values at Saint John. We consider these residues as representing the random portion of the height of high water sensed at BLB. Their distribution in time allows us to pick monthly extremes for the height deviations and establish statistics for their reccurrence. Together with an assigned frequency of tides over 29 feet they will allow us to establish return periods for extreme heights. Also the inspection of the residues will indicate times during which the levels were abnormally high. Abnormally high levels occurred on the following dates:

28-29 July 1971 24-25 August 1971 15-16 April 1972 9-10 October 1972

Tests of randomness (white noise tests) of the residues are shown (Figures 3 and 4) and indicate that raised or depressed levels have a tendency to persist.

To establish a return period we selected monthly extremes in the height residues and obtained 15 sample extremes (Table 7). Without doubt they form a poor sample because many months were not complete and some of the high waters had not been observed during that month, but it is all we have.

We plotted these values on probability paper (Figure 5) for the predicted and observed heights at Saint John. The plot for the predicted Saint John (Fig. 5a) has an outlier, which makes problematic the slope of the straight line; the plot of the observations (Fig. 5b) is somewhat less

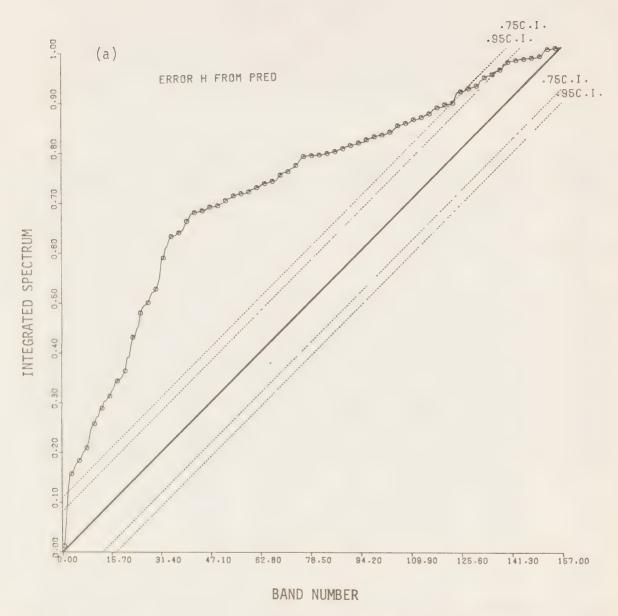


Fig. 3. White noise test on the residues obtained by regressing the predicted or observed height of high water against the height of high water at BLB. The spectrum of white noise should be flat and accumulated samples of its spectrum should fall along the solid straight line. The two dotted lines give the 75 and 95% confidence intervals for the test. It is evident that the spectrum strays from whiteness in the lower frequencies and returns towards whiteness in the higher frequencies. a) Predicted height of high water at Saint John against the height of high water at BLB. b) Observed height of high water at Saint John against the height of high water at BLB.

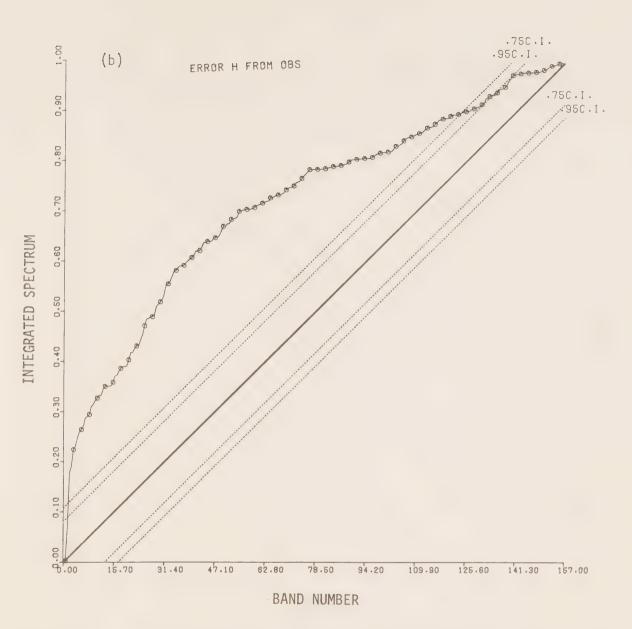


Fig. 3b.

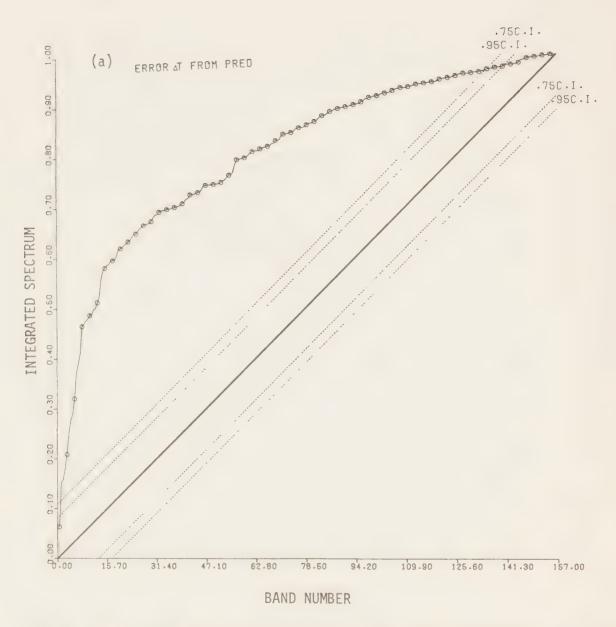


Fig. 4. White noise test on the residues in the time differences. a) From the predicted levels at Saint John. b) From the observed levels at Saint John.

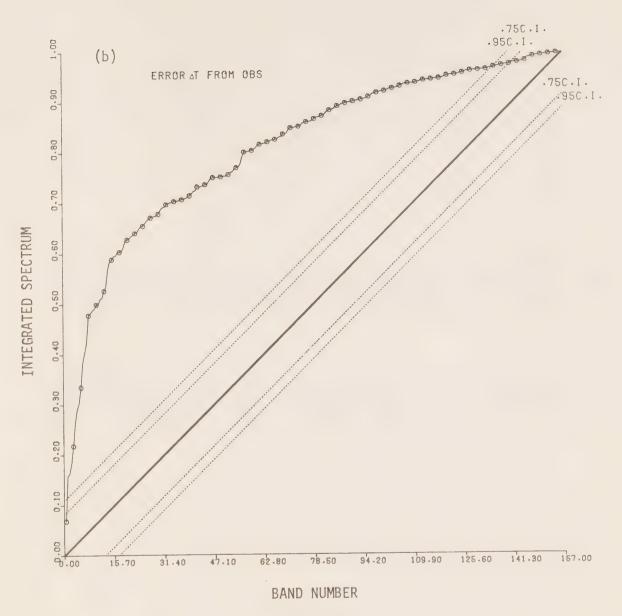
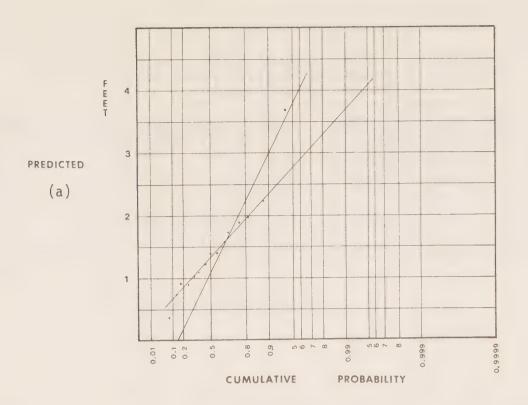


Fig. 4b.

Fig. 5. Monthly extremes in the residues in the predicted height of high water at BLB using a regression with the predicted or observed height of high water at Saint John on probability paper (double exponential distribution).

$$e^{-e^{-\frac{X-A}{B}}}$$

The slope implied by the largest extreme differs significantly from the slope implied by the remaining points (which may contain values less than the true monthly extremes because of gaps in the observations). a) Predicted height of high water at Saint John. b) Observed height at Saint John.





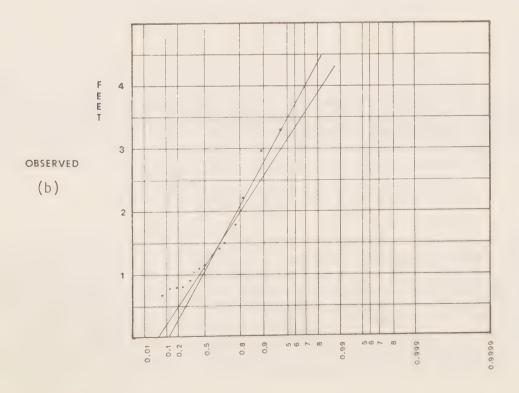


Table 7. Sample monthly extremes in the height residues at the time of high water.

Date	Residues using Observed Water Level at Saint John (ft)	Order	Residues using Predicted Tidal Height at Saint John (ft)	0r d er
04/71	.69	1	1.38	8
05	1.32	9	1.23	7
06	1.49	11	1.89	12
07	2.22	13	1.99	13
08	1.77	12	2.24	14
09	1.43	10	1.40	9
10	.78	2	.91	4
11	.80	4	.37	1
04/72	2.98	14	1.75	11
05	1.11	7	.76	2
06	.78	3	1.10	6
08	91	5	.90	3
09	1.16	8	1.02	5
10	3.29	15	3.67	15
11	1.05	6	1.60	10

uncertain. The outlier for the predictions may be due to faulty observations or to large nightly tides which had been missed during the other months because of the intermittent observations during 1971, but we have no way of checking that.

6. RETURN PERIODS AND THE CORRESPONDING HEIGHTS

The return periods in the plots are in months. For Saint John predicted, a 5 month return period falls between 2.0 and 2.2 ft depending on which slope is chosen; for the observed it falls between 2.0 and 2.1 ft. We consider the regression between Saint John and the site as giving the deterministic part of the signal: 29.1 ft at Saint John regresses to 31.1 ft of high water height at BLB for the predicted tide and 31.5 ft for the observed level. To this we add the extreme for a 5 month return period (we stick to low return periods for the residuals because the slope of the extreme is much too uncertain for high values) which is 2.0 to 2.2 ft using the predicted height and

2.0 to 2.1 ft using the observed height. The values are:

31.2 + 2.0 to 2.2 = 33.1 to 33.3 ft using the predicted height, and

31.5 + 2.0 to 2.1 = 33.5 to 33.6 ft using the observed height.

We assign a probability of once in 18 years to the 29.1 ft tide; we call it "assigned" because the tide is not a random signal. We have seen that for Saint John we have two 29.1 ft tides in recent historical time and the 100 year predictions supply four 29.1 ft tides in 100 years, although they are definitely not randomly distributed in time. In this way the compound probability of a 29.1 ft tide and a 2.0 to 2.2 ft residual is once in 90 years and 33.3 ft is the height for the highest high water at BLB with a return period of 90 years for the highest predicted tide at Saint John. The heights deduced from observed levels at Saint John are higher and we feel that their reliability is less.

A return period of 180 years supplies heights of:

31.1 + 2.4 to 3.0 = 33.5 to 34.3 ft for predicted heights,

and

31.5 + 2.5 to 2.8 = 34.0 to 34.3 ft for the observed levels.

The search for a return period of 180 years has pushed us further out in the probability curve of the extremes where the slope is very uncertain. We feel it is better to retain 33.3 ft with a return period of 90 years for an extreme height at BLB.

7. SEARCH FOR THE MONTHLY TIDE IN MINAS BASIN

The lack of whiteness of the residues suggests that they still contain a deterministic long-period signal; likely candidates for this source of variability are the semimonthly and the monthly tides which should be present in the upper reaches of the basin because of strong frictional effects. This monthly tide simply cannot be extracted from the observations at BLB, but there is some possibility that it is present in the record of Cobequid. In order to search for it we calculated the cross spectrum between the root mean square of the observed level at Saint John over 25 hours sampled hourly:

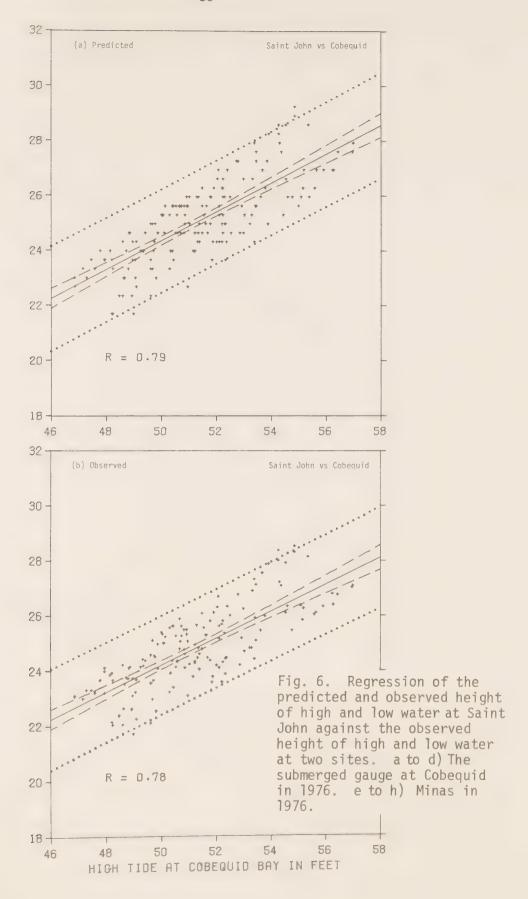
$$\overline{x} = \sqrt{\frac{\sum_{i} (x_i - x_o)^2}{25}}$$

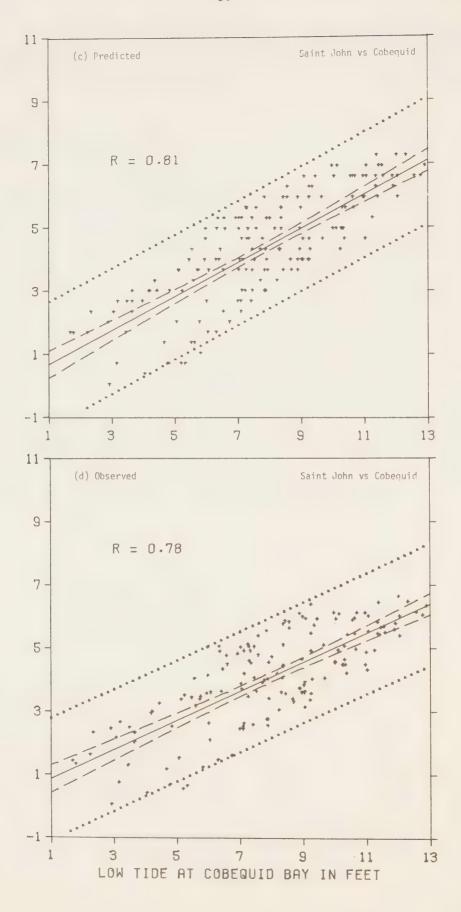
(which models the envelope of the levels at Saint John which drives the monthly tide further upstream $(x_{\hat{i}}$ is the observed level, x_0 is the mean level)) and the low pass of the level observed at Minas in 1976. The spectrum of Cobequid does indeed exhibit two marked peaks at the frequencies of 1c/27.9 d and 1c/13.9 d and the coherence of the two peaks with the mean square level at Saint John is 0.84 for the first peak and 0.97 for the second peak: they are therefore significant. However the admittance gives amplitudes of

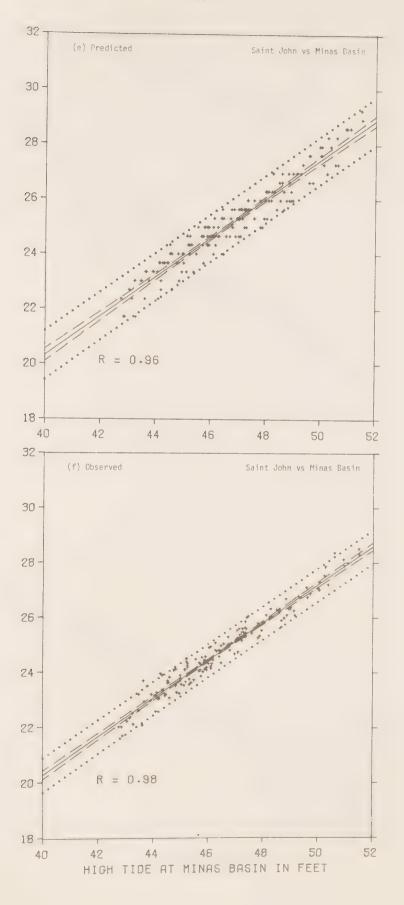
0.05 ft for the monthly signal and 0.06 ft for the semimonthly signal. We therefore have a monthly tide at the submerged gauge site, which is yet too small to be of practical significance, but there remains the possibility that it becomes much stronger at BLB.

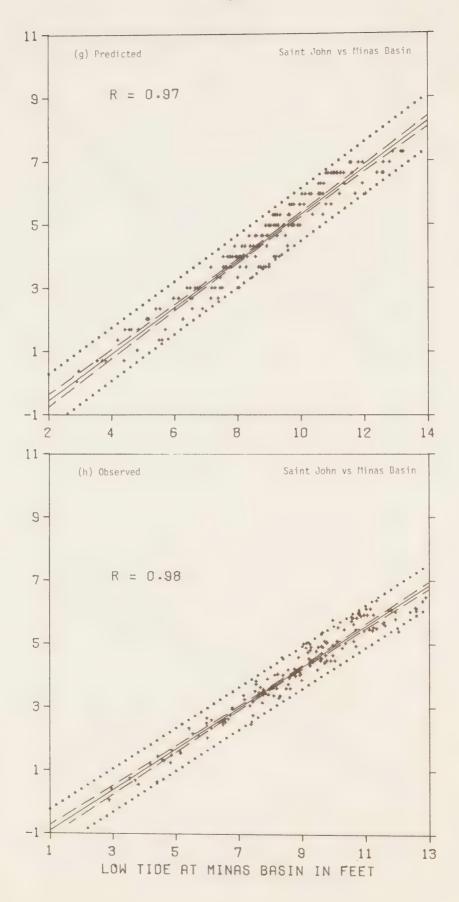
7.1 Regressions between the high and low waters at Cobequid, Minas and Saint John

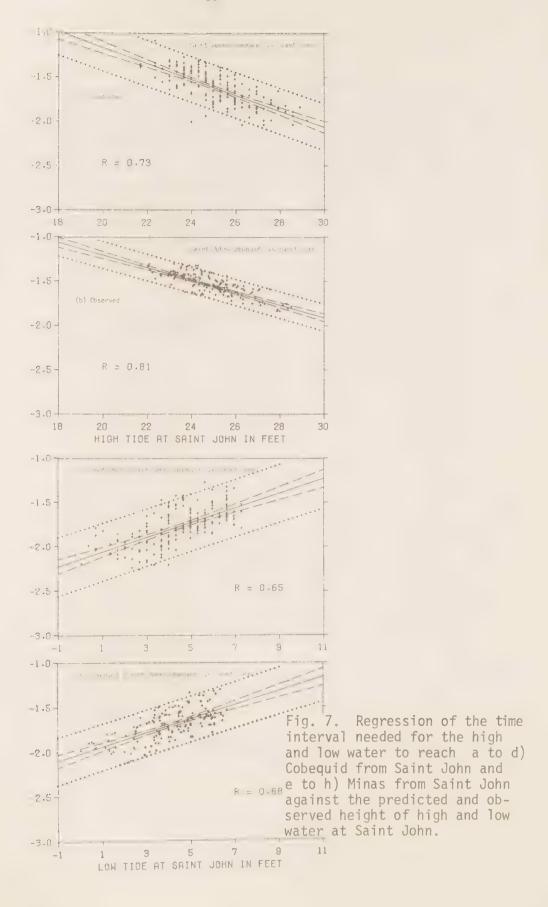
Regressions were calculated between the actual observations in 1976 on the height and time of high and low water at Minas and Cobequid and the height of high or low water at Saint John (Figures 6 and 7). We note that the regression on the heights at Minas is better with the observed level at Saint John, while at Cobequid the regression is better with the predicted at Saint John, especially low water. This confirms what we have noted at BLB, namely that events in the upper reaches of Minas Basin are probably of local origin and are not strongly coupled with those over the main body of the Bay of Fundy. Also interesting is that the time taken for high water to reach the two submerged sites increases with the height of high water (the higher the high water, the slower the wave), while the reverse holds for the low water. We recall that with respect to Salmon River, the higher the high water at Saint John, the faster it travels upstream. The change in the time of arrival of the wave at the submerged gauge sites for the observed range of high or low water at Saint John is of the order 0.5 to 0.6 h: this is quite minimal but it may help throw some light on the dynamics of the Bay of Fundy-Minas Basin system. We note finally that the scatter in the recorded heights at Cobequid is much larger than at Minas or at BLB; which suggests that some difficulties were encountered in maintaining the calibration of the instrument in situ during the observations.

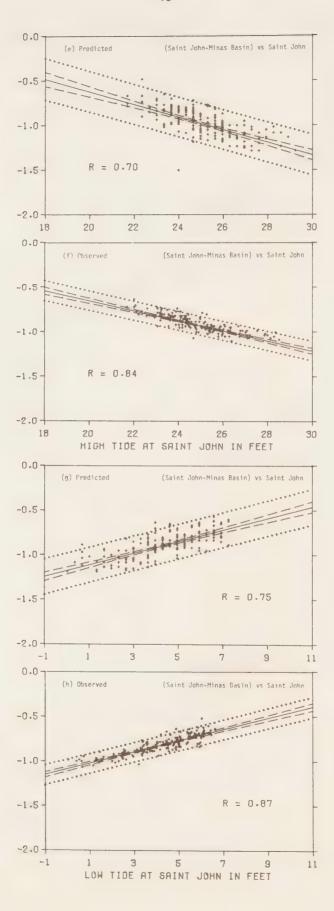












8. DISCUSSION

We attempted to obtain an estimate of the extreme level which could be reached at Board Land Bridge due to a combination of the tide and of a disturbance in Minas Basin; we neglected the possibility of a coincident flood (apparently a possibility in the spring) because such considerations lie outside our field of competence. Extreme tides (Table 6) occur mainly in the spring i.e. March, April, May, with fewer occurrences in the fall, i.e. Oct, Nov. Dec, the underlined months having a higher probability of containing an extreme. The spring extremes invariably occur around midnight while the fall extremes occur around noon. The highest tide which reaches a level of 29.1 ft at Saint John, being regressed to 31.1 ft at BLB, does not represent an extremely high level at BLB and what matters most is the possible coincidence of a weather disturbance being superimposed. The maximum value of the disturbance is not related to the tide and it may have its peak at any time before or after high water. But at BLB only high water reaches the site and we must restrict our considerations to the coincidence of high water with a given value of the disturbance. The sample of extreme residues between the value of the level at BLB regressed from Saint John and the actual level observed is taken as a measure of the disturbance present in Minas Basin at the instant of high water; it cannot be a measure of the peak value of the individual disturbance since the site chosen makes it impossible to follow the full development of surges in the basin. Like most sample of extremes it is not easy to interpret, especially because of the value of 3.67 ft which it contains for 1972; (Table 7) the point falls well off the regression curve for the other samples (Fig. 5a) and we simply cannot decide if we should keep it or reject it. Our solution to the dilemma is to pick an extreme of low probability (1/5 months) whose position is very little affected by this outlier and calculate its compound probability with the rarest tidal event. In order to understand the influence of tide heights on the probability of occurrence, we did note that the tide has an upper bound in contrast to a random variable. In the case of Saint John, the upper bound is slightly over 29.1 ft, let us say 29.2 ft. We have an extremely rapid decrease in assigned probability going from a spring tide at 26.1 ft (26.4 ft at BLB) twice a month, to a perigean tide of 26.6 ft (27.2 ft at BLR) once a month, to a spring perigean tide of 28.3 ft (29.7 ft at BLB) once every 13 months. to a declinational spring perigean tide of 29.1 ft (31.1 ft at BLB) once every 19 years to the upper bound of 29.2 ft (31.2 ft at BLB) which has a probability of near zero. It is easy to pick a tide level with an assigned value of probability. We definitely cannot do the same thing with the weather disturbances because of the shortness of the observations and the selection of the site of measurements: we did the best we could with what was available. The highest level we selected is one of the least probable although the level of probability we quoted (once in 90 years) cannot be considered as absolute in view of the rapid change in the assigned probability in the region of extreme tides. Figh levels with a higher probability could have been selected such as:

1) a perigean tide of 27.2 ft (at BLB) once a month with a disturbance of 2.2 ft (once per 8 months)

27.2 + 2.2 = 29.4 ft once per 8 months, or

2) a spring perigean tide of 29.7 ft (at BLB) with a disturbance of 2.2 ft

29.7 + 2.2 = 31.9 ft once every nine years.

hese extreme levels are lower than the one quoted in section 6 and can be eached more often. We were reluctant to use the extreme disturbance of .67 ft as it does not seem to belong. For a spring perigean tide of 29.7 ft n conjunction with such an extreme, we would have:

 $29.7 + 2.6 \rightarrow 3.7 = 32.4 \rightarrow 33.4$ ft once every 16 years.

he margin of 1.1 ft implied by the uncertainty in the fitting curve to the robability distribution makes this estimate too blunt to use.

We conclude that we have gone as far as we could in the study of the eagre set of data presently at our disposal. Some serious studies on the ynamics of Minas Basin will eventually have to be undertaken. To a good irst approximation the basin may be viewed as cut off from the Bay of Fundy s far as disturbances are concerned and the first step in such a study ould be the installation of a gauge capable of measuring the full range of he tide over at least 2 or 3 years with in addition, a network of a few dditional gauges at other sites over a shorter interval.

9. ACKNOWLEDGEMENTS

We wish to express our appreciation to Mr. F.I. Lorant of M.M. Dillon imited, Consulting Engineers and Planners, Toronto, who brought the high ater problem at Board Landing Bridge to our attention, to Dr. W.D. Forrester or many fruitful suggestions and comments, and to Mrs. Margaret Johnstone or her patience in the preparation and typing of the manuscript for ublication.

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FOR HIGH WATER AT BOARD LANDING BRIDGE, TRURO, NOVA SCOTIA

